ΑΞϹΟΜ

October 2016

Dynegy Zimmer, LLC 1781 US Route 52 Moscow, OH 45153

RE: History of Construction USEPA Final CCR Rule, 40 CFR § 257.73(c) Zimmer Power Station Moscow, Ohio

On behalf of Dynegy Zimmer, LLC, AECOM has prepared the following history of construction for the Coal Pile Runoff Pond and D Basin at the Zimmer Power Station in accordance with 40 CFR § 257.73(c). The Gypsum Recycle Pond is an incised pond with a storage volume of less than 20 acrefeet. A history of construction is not required for Gypsum Recycle Pond as specified in § 257.73(b).

BACKGROUND

40 CFR § 257.73(c)(1) requires the owner or operator of an existing coal combustion residual (CCR) surface impoundment that either (1) has a height of five feet or more and a storage volume of 20 acre-feet or more, or (2) has a height of 20 feet or more to compile a history of construction by October 17, 2016 that contains, to the extent feasible, the information specified in 40 CFR § 257.73(c)(1)(i)–(xii).

The history of construction presented herein was compiled based on existing documentation, to the extent that it is reasonably and readily available (see 80 Fed. Reg. 21302, 21380 [April 17, 2015]), and AECOM's site experience. AECOM's document review included record drawings, geotechnical investigations, pipe inspection reports, etc. for the Coal Pile Runoff Pond and D Basin at the Zimmer Power Station.



HISTORY OF CONSTRUCTION

§ 257.73(c)(1)(i): The name and address of the person(s) owning or operating the CCR unit; the name associated with the CCR unit; and the identification number of the CCR unit if one has been assigned by the state.

Operator:	Dynegy Zimmer, LLC
Address:	1500 Eastport Plaza Drive Collinsville, IL 62234
CCR Units:	Coal Pile Runoff Pond, ODNR Dam ID No. 8741-010, NID ID No. OH01393 D Basin

The D Basin does not have a state assigned identification number.

§ 257.73(c)(1)(ii): The location of the CCR unit identified on the most recent USGS $7^{1}/_{2}$ or 15 minute topographic quadrangle map or a topographic map of equivalent scale if a USGS map is not available.

The locations of the Coal Pile Runoff Pond and D Basin have been identified on an USGS 7-1/2 minute topographic quadrangle map in **Appendix A**.

§ 257.73(c)(1)(iii): A statement of the purpose for which the CCR unit is being used.

The following captures the purpose of the CCR unit:

- The Coal Pile Runoff Pond is being used to clarify D Basin discharge water, FGD waste streams, landfill leachate water, and other non-CCR wastewater.
- The D Basin is being used to dewater dredged CCR and non-CCR material from other ponds. The dewatered material is excavated and transported to the Zimmer landfill.

§ 257.73(c)(1)(iv): The name and size in acres of the watershed where the CCR unit is located.

The Coal Pile Runoff Pond and D Basin are located in the Little Indian Creek-Ohio River Watershed with a 12-digit Hydrologic Unit Code (HUC) of 050902011107 with a drainage area of 15,657 acres (USGS, 2016).



§ 257.73(c)(1)(v): A description of the physical and engineering properties of the foundation and abutment materials on which the CCR unit is constructed.

The foundation materials for the Coal Pile Runoff Pond and D Basin consists of, from top to bottom, fine grained alluvial soils, coarse grained soils, and bedrock. The physical properties of the fine grained alluvial soil are described as soft to stiff, moist, lean clay. The coarse grained foundation soils are described as medium dense to very dense, fine sand, silty sand, and gravelly sand. Portions of the fine grained soil and coarse grained soil are separated by a layer comprised of silty clayey sand and silt. The bedrock is described as limestone. An available summary of the engineering properties of the foundation materials is presented in **Table 1** below. The engineering properties are based on previous geotechnical explorations and laboratory testing.

Material	Unit Weight (pcf)	Effective (drained) Shear Strength Parameters		Total (undrained) Shear Strength Parameters	
	. ,	c' (psf)	Φ' (°)	c (psf)	Φ (°)
Alluvial Clays	128	200	30	600	16
Silty Clayey Sand	128	150	35	400	16
Fine Sand, Silty Sand, Gravelly Sand	120	0	31	0	31
Limestone	140	Impenetrable			

Table 1. Summary of Foundation Material Engineering Properties

The Coal Pile Runoff Pond and D Basin are enclosed impoundments with embankments and do not have abutments.

§ 257.73(c)(1)(vi): A statement of the type, size, range, and physical and engineering properties of the materials used in constructing each zone or stage of the CCR unit; the method of site preparation and construction of each zone of the CCR unit; and the approximate dates of construction of each successive stage of construction of the CCR unit.

The construction of the Coal Pile Runoff Pond included a 3-foot thick compacted clay liner. The physical and engineering properties for the clay liner are not reasonably and readily available. Physical properties of the embankment materials for the Coal Pile Runoff Pond are described as very stiff to hard fine grained material typically classified as sandy lean clay. An available summary of the engineering properties of the embankment construction materials is presented in **Table 2** below. The engineering properties are based on previous geotechnical explorations and laboratory testing.

D Basin was constructed over a portion of the historical coal storage area. The historical coal storage area was originally constructed as an enclosed impoundment with embankments and lined with a 3-foot thick compacted clay liner. Common fill was placed at a variable thickness over the historical clay liner to raise the subbase elevation for D Basin construction.



Construction of the D Basin typical liner section consisted of (from bottom to top) a 1-foot thick new clay liner layer with a designed permeability of 1x10⁻⁷ cm/s, a geotextile fabric, a 1-foot thick granular material underdrain layer with 6-inch diameter (dia.) high density polyethylene (HDPE) underdrain piping, a second geotextile fabric, and a 2-foot thick bottom ash protection layer. Physical properties of the embankment material typically classified as silty clayey sand with gravel and silty sand with gravel. An available summary of the engineering properties of the embankment construction materials is presented in **Table 2** below. The engineering properties are based on previous geotechnical explorations and laboratory testing. The physical and engineering properties for the other above-mentioned liner materials are not reasonably and readily available.

Material	Unit Weight (pcf)	Effective (drained) Shear Strength Parameters		Total (undrained) Shear Strength Parameters	
	. ,	c' (psf)	Φ' (°)	c (psf)	Φ (°)
Sandy Lean Clay (CL)	128	50	30	225	20
Silty Clayey Sand with Gravel	127	0	30	0	30

 Table 2. Summary of Embankment Construction Material Engineering Properties

The method of site preparation and construction for the Coal Pile Runoff Pond is not reasonably and readily available. Site preparation and construction of the D Basin were completed in accordance with the *Dewatering Basin "D"* record drawings (presented in **Appendix B**).

The approximate dates of construction of each successive stage of construction of the Coal Pile Runoff Pond and D Basin are provided in **Table 3** below.

Date	Event
Prior to 1985	Development of historical impoundment
1986-1987	Conversion of historical impoundment to coal pile storage area
1987	Construction of Coal Pile Runoff Pond
2003	Construction of D Basin over portion of coal pile storage area

Table 3. Approximate dates of construction of each successive stage of construction.

§ 257.73(c)(1)(vii): At a scale that details engineering structures and appurtenances relevant to the design, construction, operation, and maintenance of the CCR unit, detailed dimensional drawings of the CCR unit, including a plan view and cross sections of the length and width of the CCR unit, showing all zones, foundation improvements, drainage provisions, spillways, diversion ditches, outlets, instrument locations, and slope protection, in addition to the



normal operating pool surface elevation and the maximum pool surface elevation following peak discharge from the inflow design flood, the expected maximum depth of CCR within the CCR surface impoundment, and any identifiable natural or manmade features that could adversely affect operation of the CCR unit due to malfunction or mis-operation.

Drawings that contain items pertaining to the requested information for the Coal Pile Runoff Pond and D Basin are listed in **Table 4** below. Items marked as "Not Available" are items not found during a review of the reasonably and readily available record documentation.

Table 4. List of drawings containing items pertaining to the information requested in § 257.73(c)(1)(vii).

(•1).	Coal Pile Runoff Pond	D Basin
Dimensional plan view (all zones)	1-30250	1-30250B, 1-30250E
Dimensional cross sections	1-30253	1-30250C, 1-30250D
Foundation Improvements	1-30253	1-30250C, 1-30250D, 1-30250H
Drainage Provisions	Not Applicable	1-30250C to 1-30250E, 1-5434-01 to 1-5434-03
Spillways and Outlets	Not Available	1-30250B, 1-30250F, 1-30250G 1-5434-01 to 1-5434-03
Diversion Ditches	Not Applicable	Not Applicable
Instrument Locations	Plate 2	Fig. No. 2A
Slope Protection	Not Available	1-30250C, 1-30250D, 1-30250H
Normal Operating Pool Elevation	1-30253	1-30250G
Maximum Pool Elevation	Not Available	1-30250G
Approximate Maximum Depth of CCR in 2016	16 feet	8 feet

All drawings referenced in **Table 4** above can be found in **Appendix B** and **Appendix C**.



Based on the review of the drawings listed above, no natural or manmade features that could adversely affect operation of these CCR units due to malfunction or mis-operation were identified.

§ 257.73(c)(1)(viii): A description of the type, purpose, and location of existing instrumentation.

Existing instrumentation within the Coal Pile Runoff Pond includes a vibrating-wire piezometer installed in 2014. The purpose of the piezometer is to measure the pore water pressure within the embankment. A location map of the existing piezometer instrumentation (B-WW-1401) is shown on Plate 2 presented in **Appendix C**.

Existing instrumentation within the D Basin include open-standpipe piezometers installed in 2015 near the western embankment. The purpose of the piezometers is to measure the pore water pressures at the crest and toe of the outer embankment. There are two (2) piezometers adjacent to D Basin. A location map of the existing piezometer instrumentation is shown on Figure 2A presented in **Appendix C**.

§ 257.73(c)(1)(ix): Area-capacity curves for the CCR unit.

Area-capacity curves for the Coal Pile Runoff Pond and D Basin are not reasonably and readily available.

§ 257.73(c)(1)(x): A description of each spillway and diversion design features and capacities and calculations used in their determination.

The Coal Pile Runoff Pond contains two (2) 15-inch diameter (dia.) high density polyethylene (HDPE) pipe culverts located in the northeast corner of the impoundment. The pipe culverts drain into the adjacent Wastewater Pond. In 2016, the discharge capacity of the Coal Pile Runoff Pond was evaluated using HydroCAD 10 software modeling a 1,000-year, 24-hour rainfall event. The model results indicate that the Coal Pile Runoff Pond spillway has enough water storage capacity and will not overtop the embankment during the 1,000-year, 24-hour storm event. The results of the HydroCAD 10 analysis are presented below in **Table 5**.

D Basin drains into a reinforced concrete sump structure that contains two sump pumps. The lead pump activates when the sump liquid level reaches El. 485.2 feet and the lag pump activates when the sump fluid level reaches El. 486.7 feet. The minimum pump liquid level, or shut-off level, is at El. 481.7 feet. Both pumps drain into separate 6-inch dia. HDPE pipes that discharge into the Coal Pile Runoff Pond. D Basin also contains a 10-foot-wide emergency overflow spillway with an invert elevation of 504 feet. The overflow spillway drains into the adjacent C Basin. Unless otherwise mentioned, all elevations in this report are on datum NAVD88.

In 2016, the discharge capacity of D Basin was evaluated using HydroCAD 10 software modeling a 1,000-year, 24-hour rainfall event. During the modeled event, the Ohio River flood pool would back up into the D Basin through the outfall structures from the Clearwater Pond and from emergency outfall structures for the A, B, and C Basins. During this scenario,



the Wastewater Pond Complex would be flooded to an elevation of 506 feet, and the D Basin would be part of a continuous pool that is also comprised of the A, B, and C Basin. The model results indicated that the Wastewater Pond Complex had enough storage capacity above the Ohio River flood pool level and will not overtop the perimeter embankment during the 1,000-year, 24-hour storm event. The results of the HydroCAD 10 analysis are presented below in **Table 5**.

	Coal Pile Runoff Pond	Combined Basins (A, B, C & D)
Approximate Minimum Berm Elevation ¹ (ft)	509.2	509.0
Approximate Emergency Spillway Elevation ¹ (ft)	Not Applicable	Not Applicable
Starting Pool Elevation ¹ (ft)	507.9	506.0
Peak Elevation ¹ (ft)	509.0	506.8
Time to Peak (hr)	12.2	13.0
Surface Area (ac)	2.5	27.3
Storage ² (ac-ft)	2.8	20.8

Table 5. Results of HydroCAD 10 analysis

Note: 1. Elevations are based on NAVD88 datum

2. Storage given is from Starting Pool Elevation to Peak Elevation.

§ 257.73(c)(1)(xi): The construction specifications and provisions for surveillance, maintenance, and repair of the CCR unit.

The construction specifications for the Coal Pile Runoff Pond are not reasonably and readily available. Available construction specifications for D Basin are outlined on Drawing 1-30250H presented in **Appendix B**.

The operations and maintenance plans for Coal Pile Runoff Pond and D Basin are currently being prepared by Dynegy Zimmer, LLC.



§ 257.73(c)(1)(xii): Any record or knowledge of structural instability of the CCR unit.

There is no record or knowledge of structural instability of the Coal Pile Runoff Pond and D Basin at Zimmer Power Station.

LIMITATIONS

The signature of AECOM's authorized representative on this document represents that to the best of AECOM's knowledge, information and belief in the exercise of its professional judgment, it is AECOM's professional opinion that the aforementioned information is accurate as of the date of such signature. Any recommendation, opinion or decisions by AECOM are made on the basis of AECOM's experience, qualifications and professional judgment and are not to be construed as warranties or guaranties. In addition, opinions relating to environmental, geologic, and geotechnical conditions or other estimates are based on available data and that actual conditions may vary from those encountered at the times and locations where data are obtained, despite the use of due care.

Sincerely,

Claudia Frack

Claudia Prado Project Manager

Value Mola

Victor Modeer, P.E., D.GE Senior Project Manager

REFERENCES

United States Environmental Protection Agency (USEPA). (2015). *Hazardous and Solid Waste Management System; Disposal of Coal Combustion Residuals from Electric Utilities; Final Rule.* 40 CFR Parts 257 and 261, 80 Fed. Reg. 21302, 21380 April 17, 2015.

United States Geological Survey (USGS). (2016). The National Map Viewer. http://viewer.nationalmap.gov/viewer/. USGS data first accessed in March of 2016.

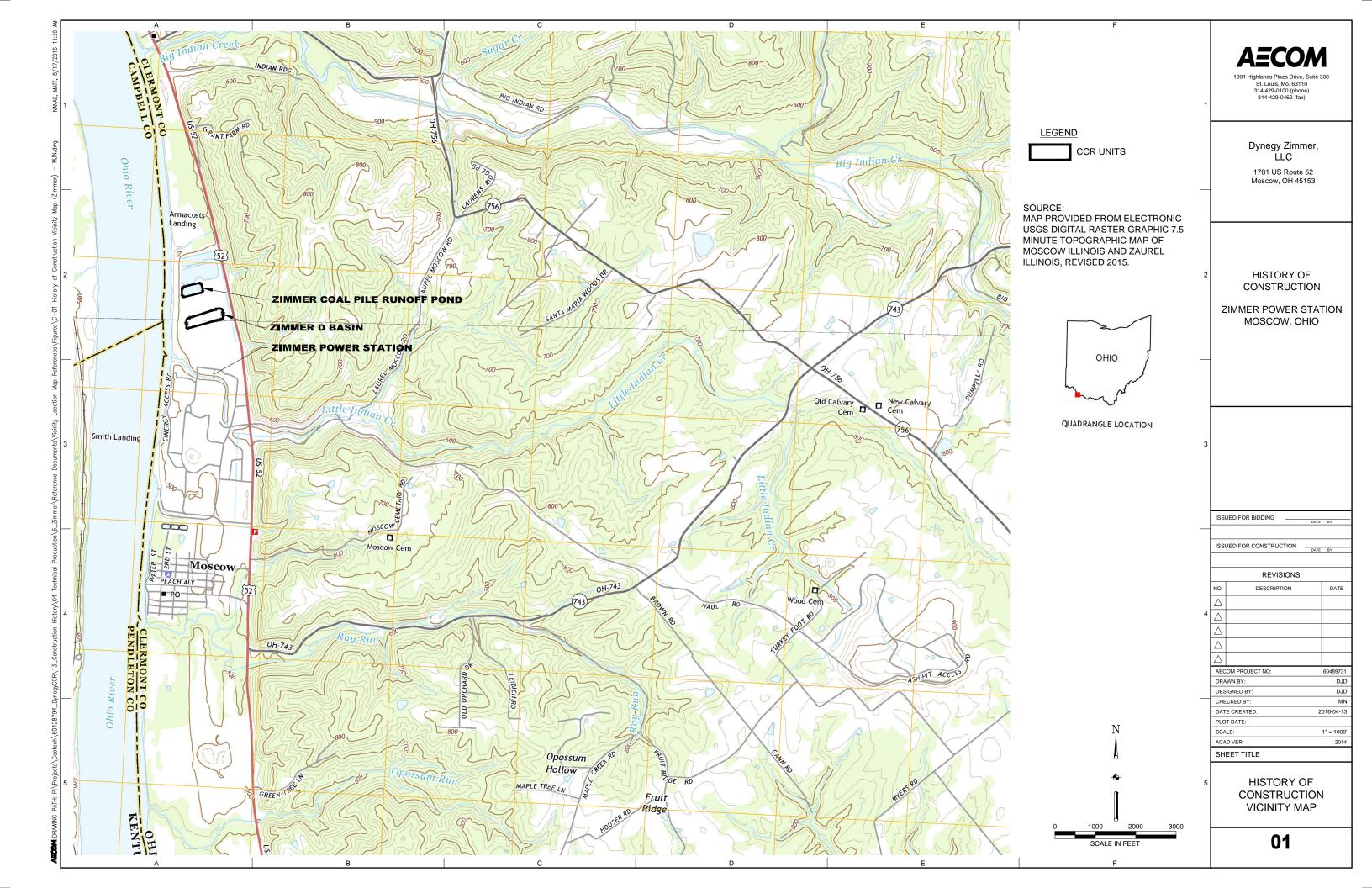
APPENDICES

Appendix A: History of Construction Vicinity Map Appendix B: Zimmer Power Station Drawings Appendix C: Zimmer Power Station Boring and Piezometer Location



Appendix A: History of Construction Vicinity Map

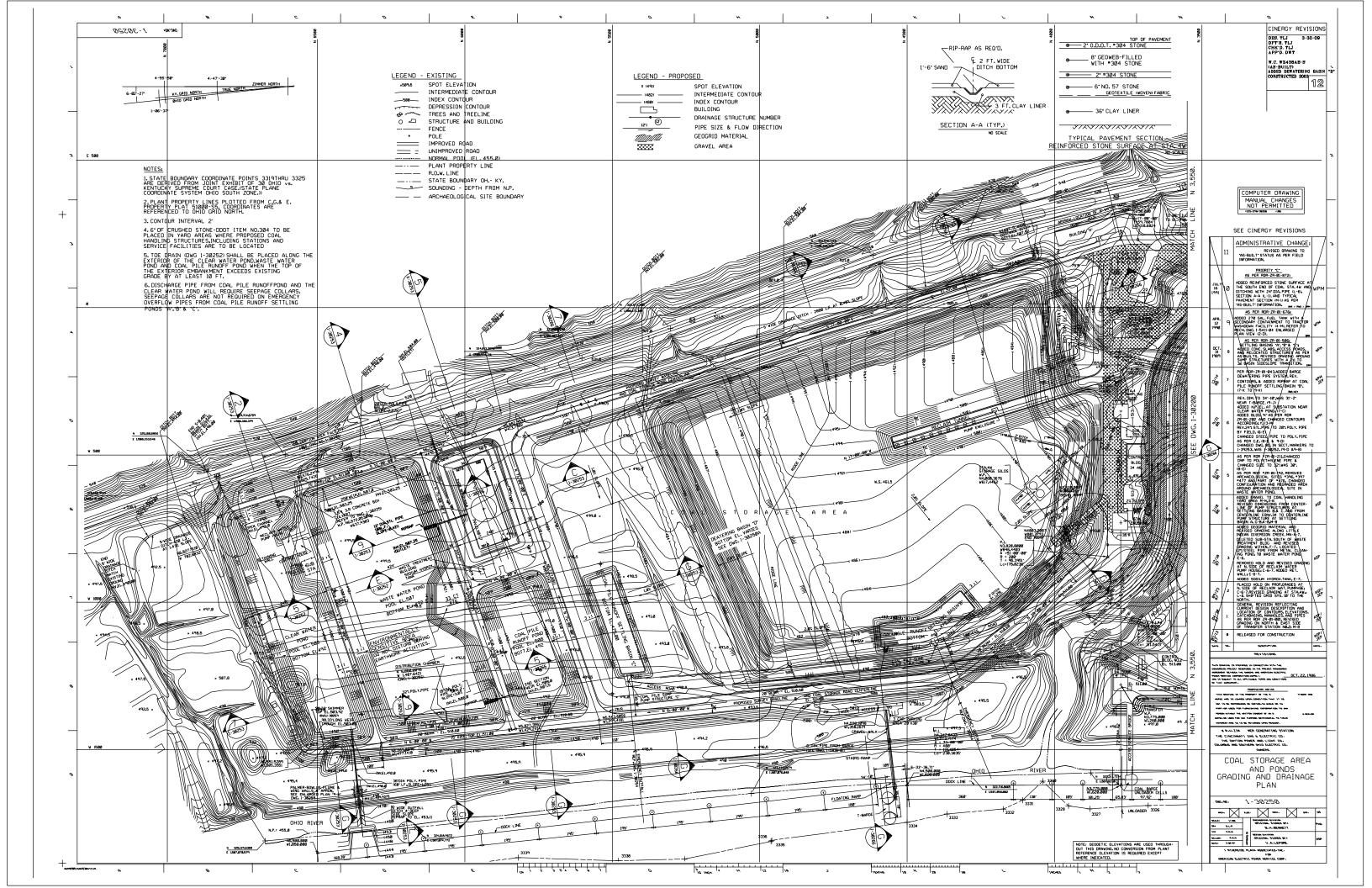
Zimmer Power Station – History of Construction §257.73(c)

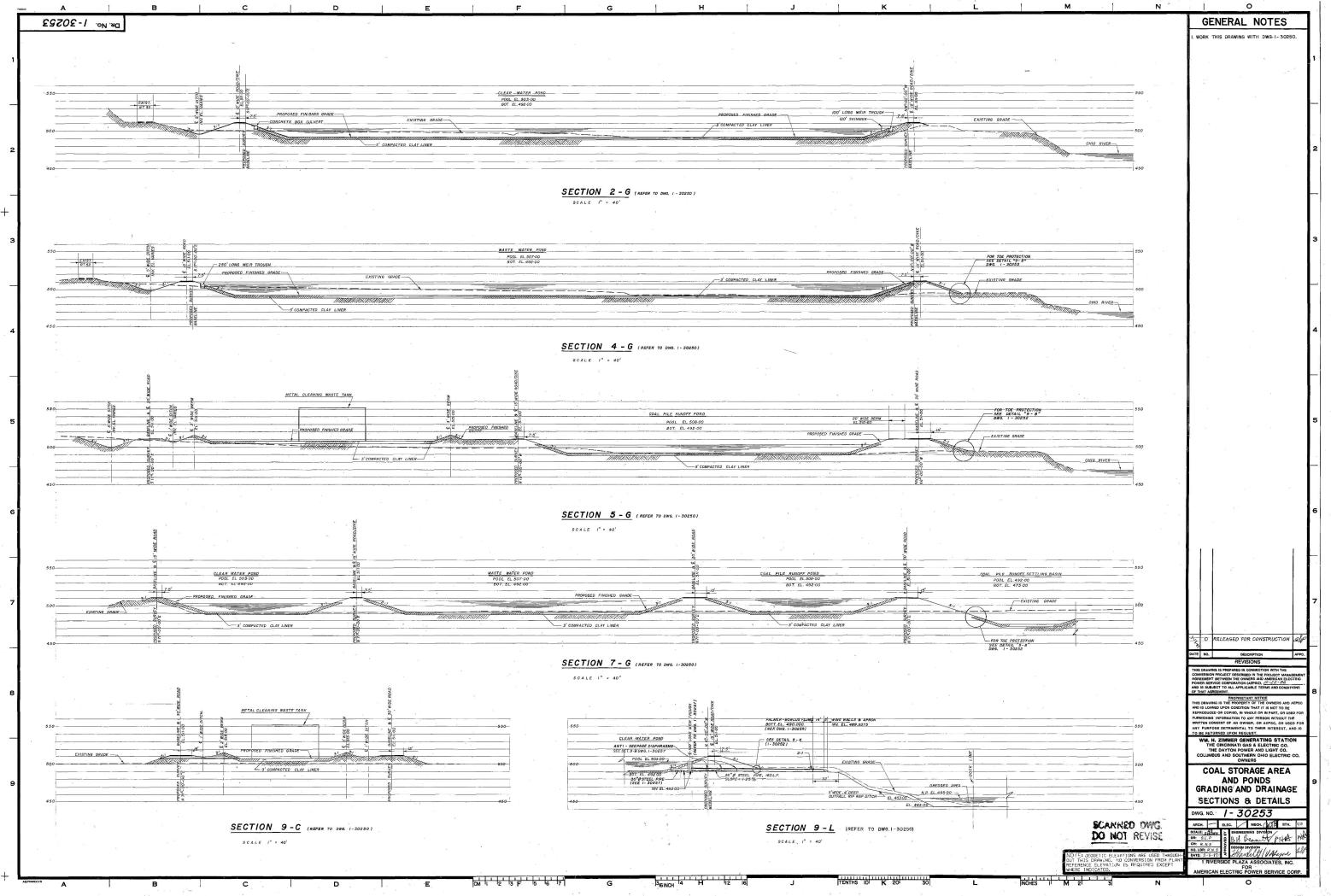


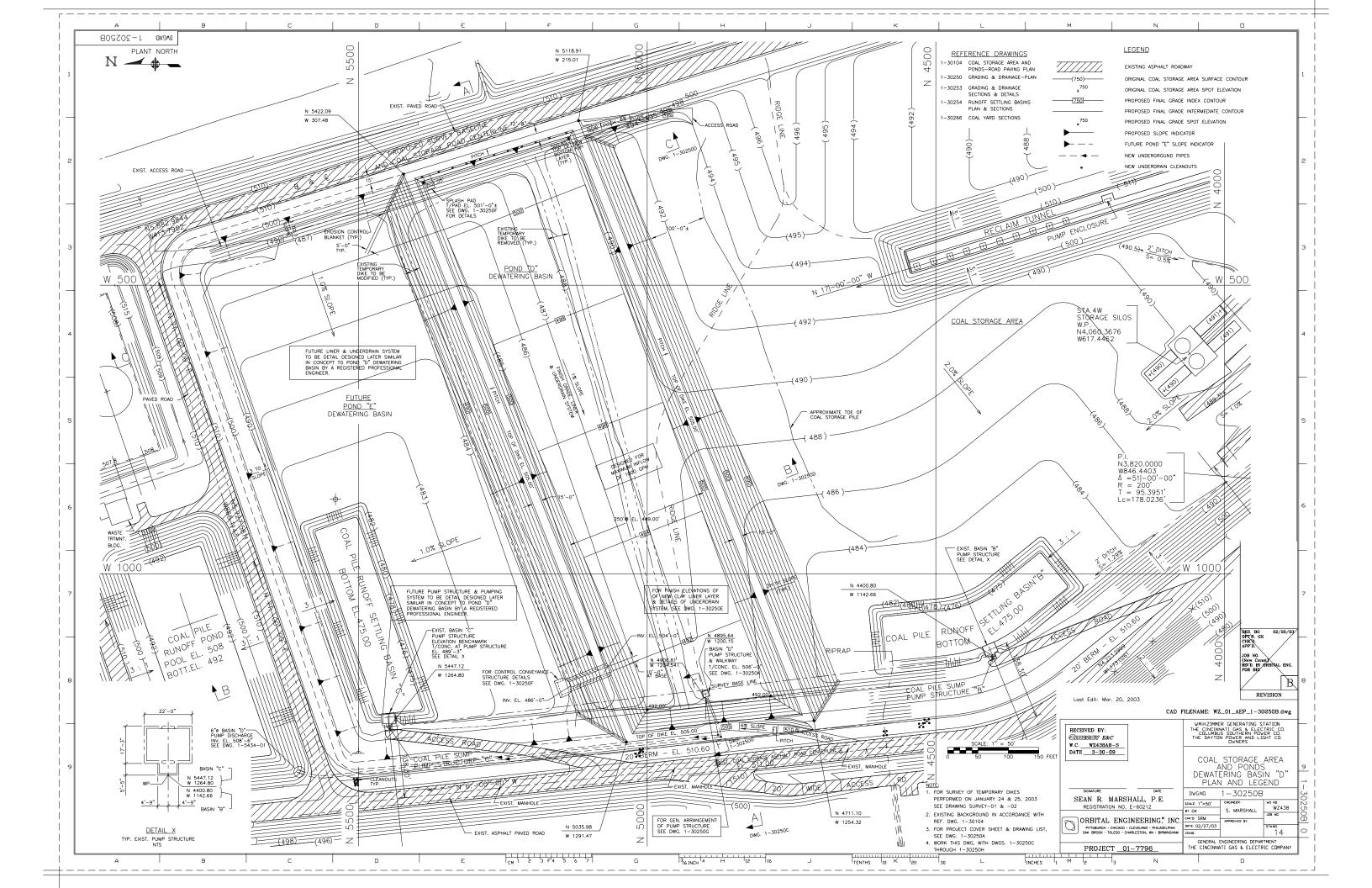
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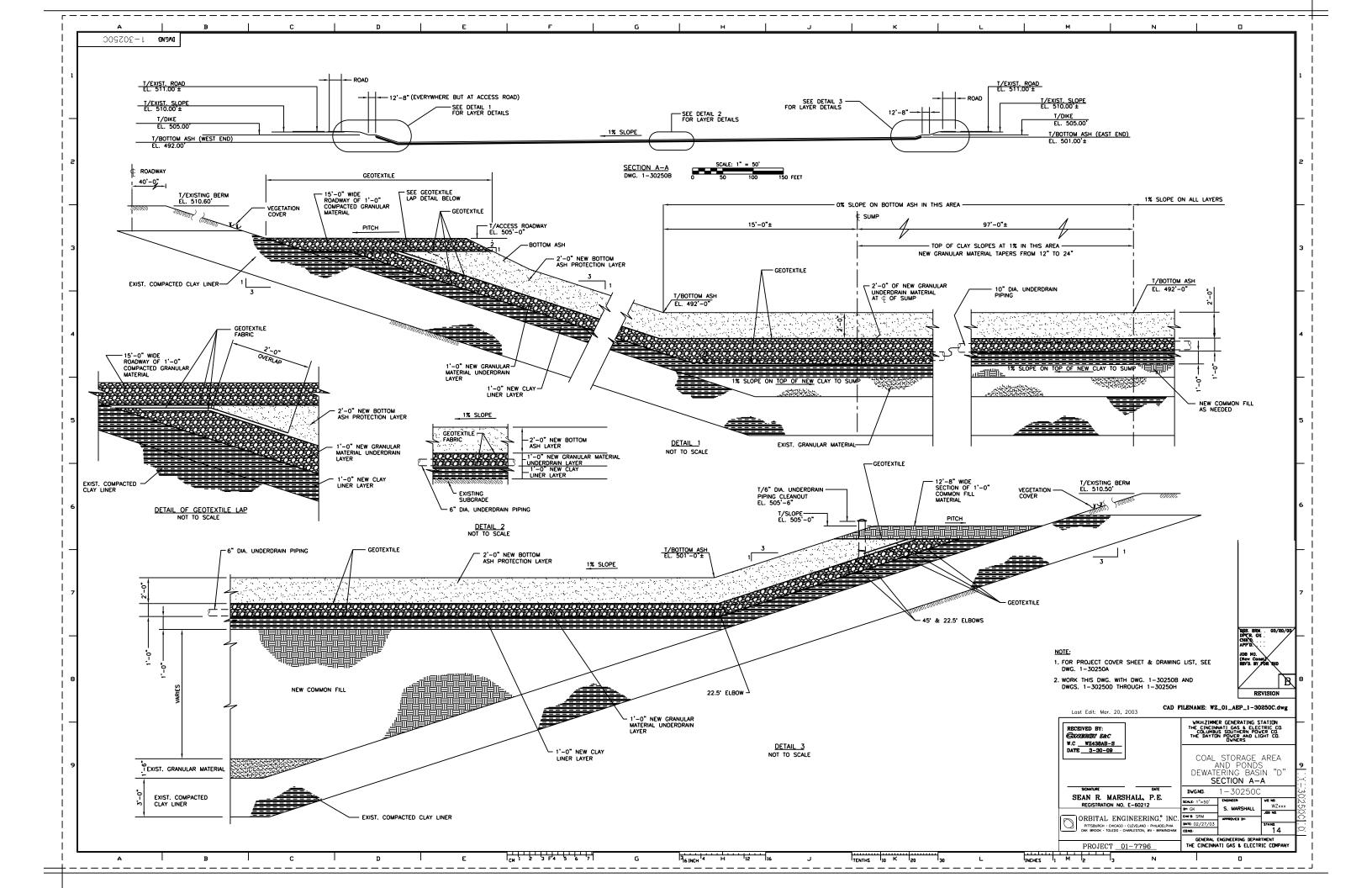
Appendix B: Zimmer Power Station Drawings

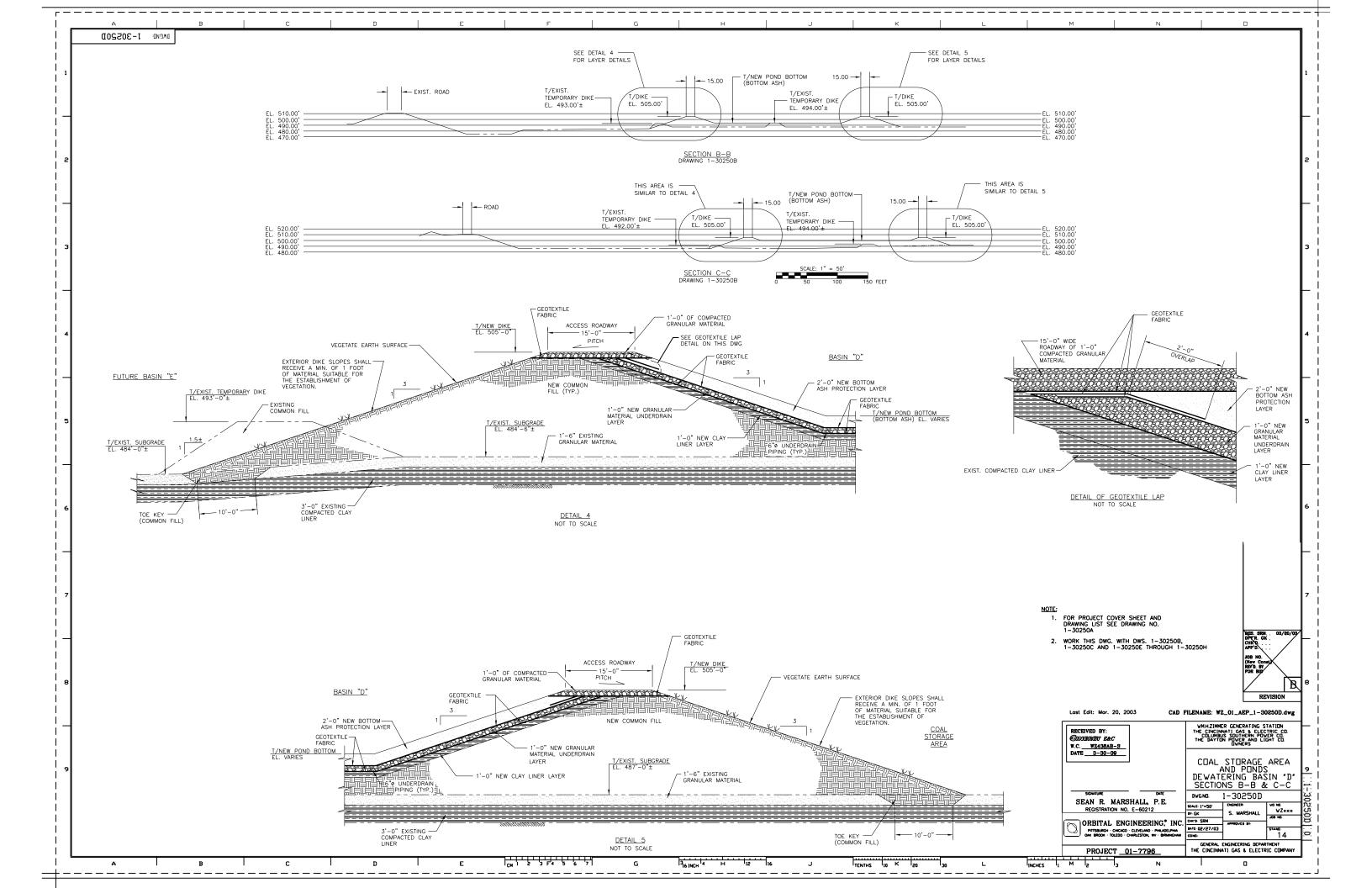
- 1. "Coal Storage Area and Ponds, Grading and Drainage Plan", Drawing No. 1-30250, Revision 13, 6 March, 1987, American Electric Power Service Corp.
- 2. "Coal Storage Area and Ponds, Grading and Drainage Sections & Details", Drawing No. 1-30253, Revision 0, 13 March, 1987, American Electric Power Service Corp.
- 3. "Coal Storage Area and Ponds, Dewatering Basin "D" Plan and Legend", Drawing No. 1-30250B, 27 February, 2003, Orbital Engineering, Inc.
- 4. "Coal Storage Area and Ponds, Dewatering Basin "D" Section A-A", Drawing No. 1-30250C, 27 February, 2003, Orbital Engineering, Inc.
- 5. "Coal Storage Area and Ponds, Dewatering Basin "D" Sections B-B & C-C", Drawing No. 1-30250D, 27 February, 2003, Orbital Engineering, Inc.
- 6. "Coal Storage Area and Ponds, Dewatering Basin "D" Liner and Underdrain Plan", Drawing No. 1-30250E, 27 February, 2003, Orbital Engineering, Inc.
- 7. "Coal Storage Area and Ponds, Dewatering Basin "D" Misc. Sections & Details", Drawing No. 1-30250F, 27 February, 2003, Orbital Engineering, Inc.
- 8. "Coal Storage Area and Ponds, Dewatering Basin "D" G.A. of Pump Pit", Drawing No. 1-30250G, 27 February, 2003, Orbital Engineering, Inc.
- 9. "Coal Storage Area and Ponds, Dewatering Basin "D" General Notes", Drawing No. 1-30250H, 27 February, 2003, Orbital Engineering, Inc.
- 10. "Coal Storage Area and Ponds, Dewatering Basin "D" Pump Pit Piping Plan", Drawing No. 1-5434-01, 20 February, 2003, Orbital Engineering, Inc.
- 11. "Coal Storage Area and Ponds, Dewatering Basin "D" Piping Sect. A-A & Det.", Drawing No. 1-5434-02, 20 February, 2003, Orbital Engineering, Inc.
- 12. "Coal Storage Area and Ponds, Dewatering Basin "D" Pump Pit Misc. Details", Drawing No. 1-5434-03, 20 February, 2003, Orbital Engineering, Inc.

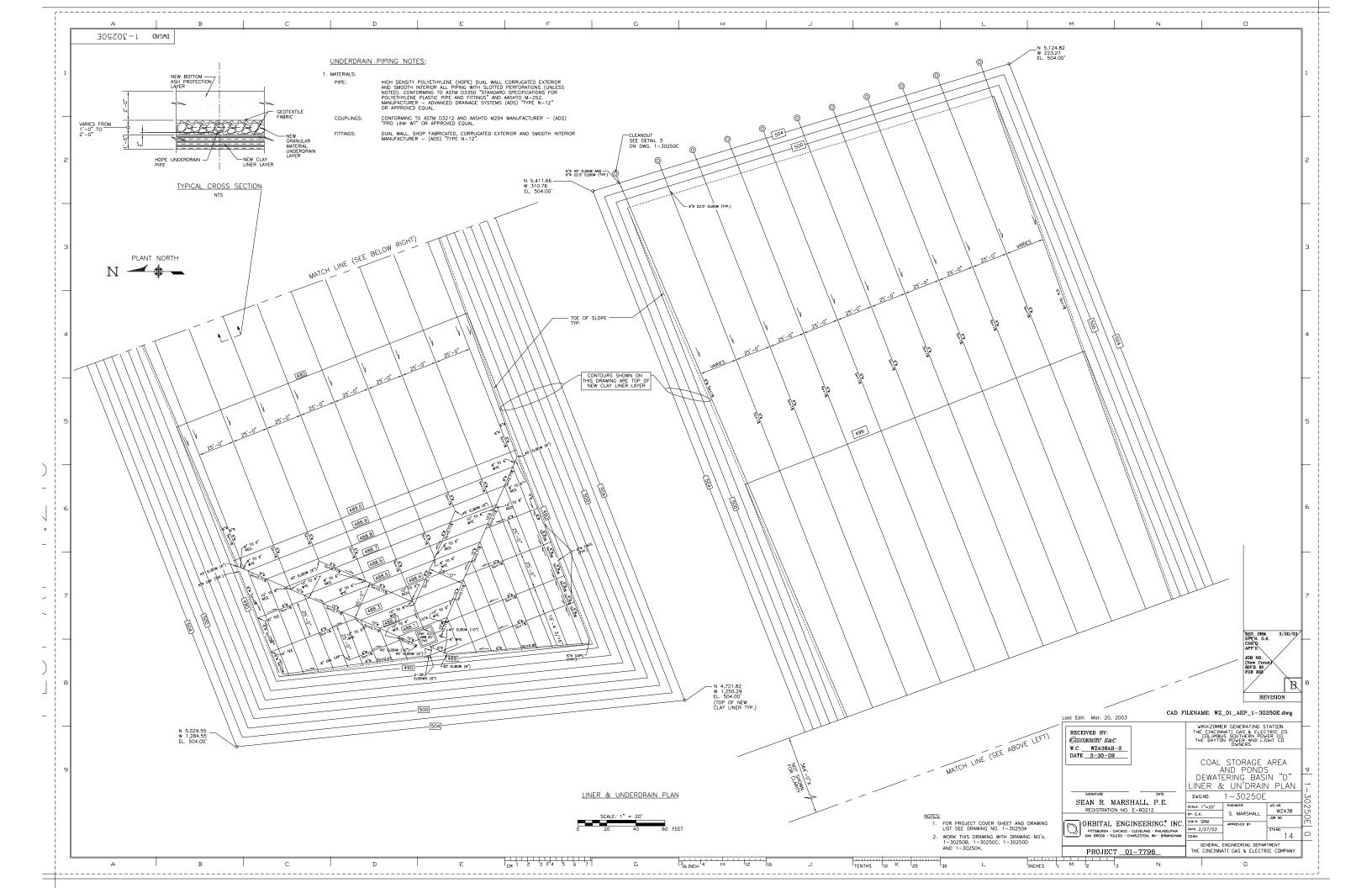


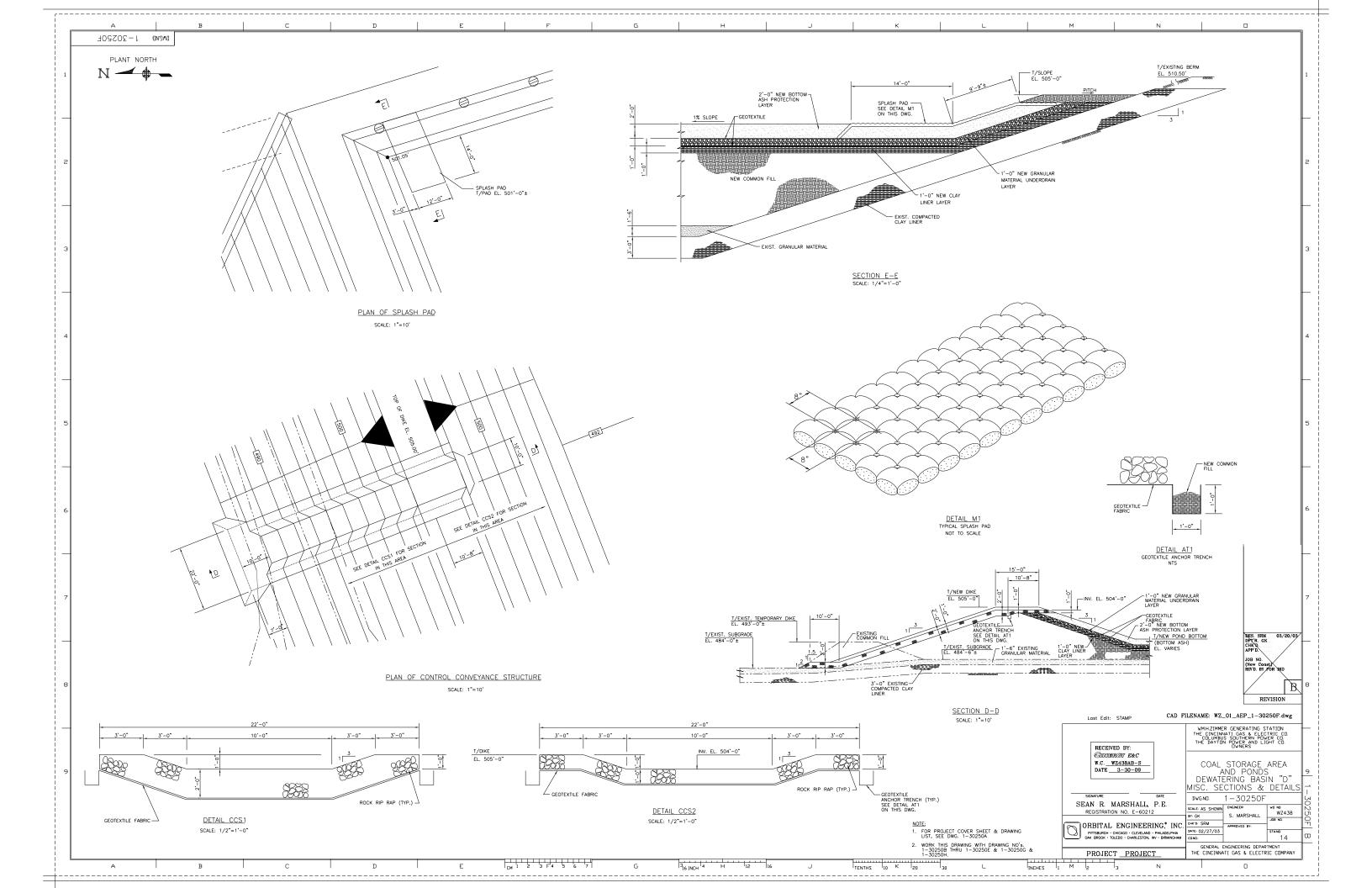


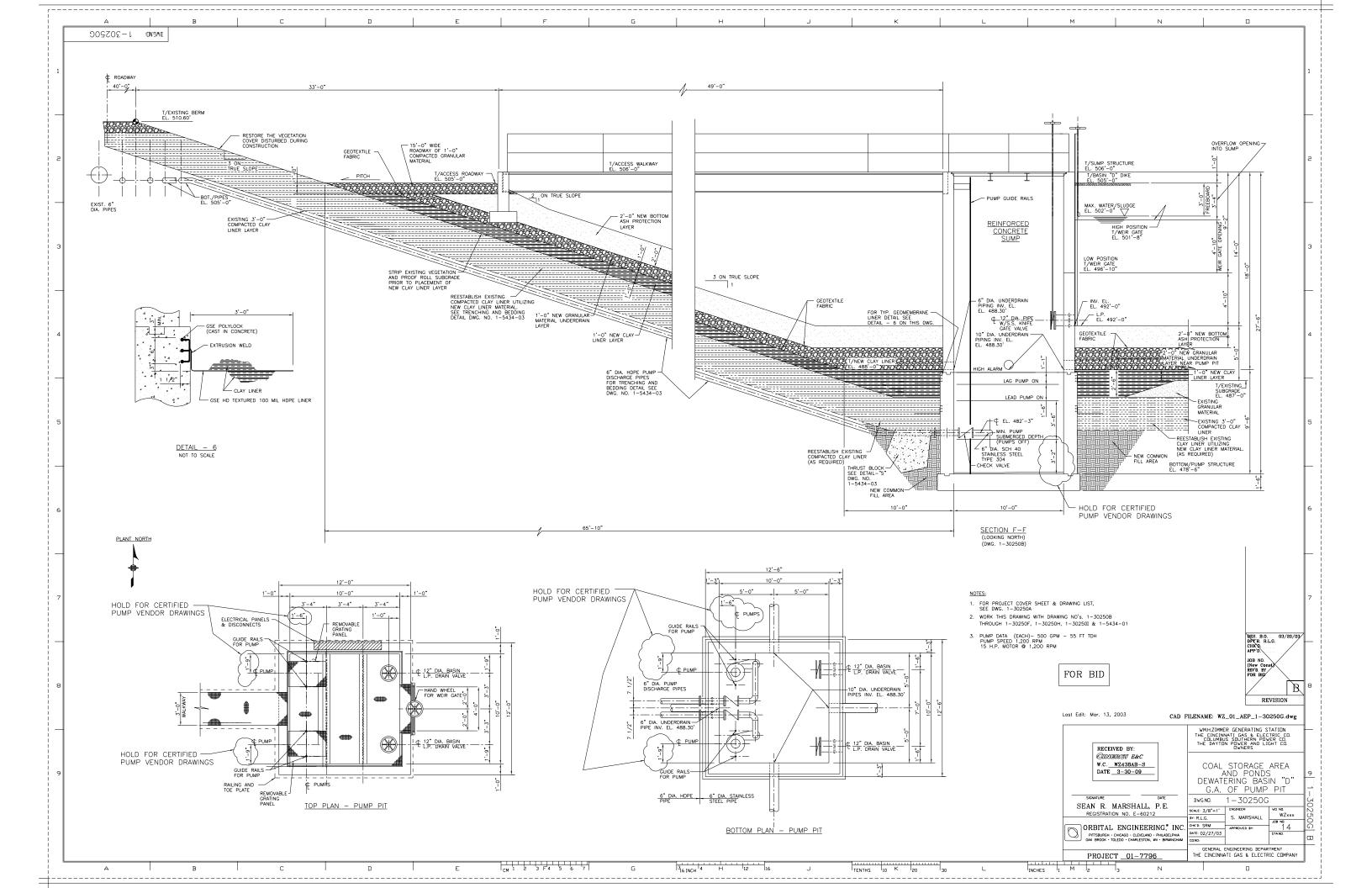












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GENERAL NOTES

- All elevations are Geodetic and are given in feet.
 The coordinate grids indicated on the drawings are Plant coordinates.
 Figured dimensions take precedence over scaled measurements.
 All work shall be performed in accordance with all applicable Federal, State and Local lows and ordinances, and shall comply with applicable regulations of all Federal. State and Local lows the measures and shall comply with applicable regulations of all Federal. State and Local lows
 Where measures applicable regulations or a state or referred to, the latest or current entition shall, applicable regulations or context or referred to.
- governmentation upenues norming jurisduction over the activity.
 Where codes, standards and specifications are cited or referred to, the latest or current edition shall apply.
 The Contractor shall be responsible for control of all water from the working areas. Storm water shall be conducted to areas away from the work in a manor to prevent erosion in accordance with the owners requirements. Storm water shall not be permitted to accumulate in the excavation or low areas of the site. The Contractor shall provide all temporary diktes, berrins, grading, sump and pumps required for temporary dewatering. All Runoff shall be directed into existing Coal Pile Runoff Settling Basins.
 The Contractor shall be perform all excavations in accordance with OSHA 29 CFR part 1926, subpart P Excavations. The Contractor shall design, furnish, install and maintain all shoring and bracing systems as required for the performance of the work.
 All excavated organic material and topsai shall be segregated from other common material to be used for earthfull. Organic material and topsai shall be stockpiled onsite as directed by the Owner for use as the vegetative layer.
 Inactive stockpile or onsite borrow areas shall be temporarily seeded in accordance with the vegetation specifications.

- Ceneral Fill Placement

 The Owner shall Contract for Third party Geotechnical Field Engineering and Laboratory Services required for Quality Control and Assurance of the Fill placement, compaction and gradations specifications. The Contractor may phere and the optimised Engineering and Laboratory Services required at the ontime meta-shall Field Engineering and Laboratory Services required at
- placement, compaction and gradations' specifications. The Contractor may provide any Geotechnical Field Engineering and Laboratory Services required at their option.
 12. Subgrade preparation: All areas receiving fill placement shall be cleared of existing fill placement, sludge/sediment deposits and other debris down to the original Coal Storage Area ground surface contours as indicated on Drawing No. 1-302508. Existing permonent dike slopes shall be stripped of vegetation in the area of work. All Subgrade surfaces shall be proof-rolled with heavy equipment until there is no visible movement beneath the equipment. Any soft, wet, unstable or otherwise with location appropriate will compacted fill material. If removal of any portions of the existing 3'-O clay liner is necessary, the liner shall be reestablished utilizing new clay liner material.
 1.3 Only soil materials conforming specifically to these specifications and in location as indicated on the drawings may be used.
 1.4 At no time shall the Contractor place any fill material. If removal of any portions, standing wet, subject and the science of the sisting, standing the contractor place any fill materials overtop of roganic material, topsoil, vegetation, standing water, mud, debris, waste sediments or sludge. If any of these materials are present they shall be removed and disposed of in a segregated method in the designated onsite area as directed by the Owner.
 1.6 No soil material shall be placed or compacted during weather conditions which would interfere with the materian, as application, as applicate, as applic

- directions. The re-compacted area shall be re-rescue for companying the formation shall (Protection Layer)
 2.1 All linert Bottom Ash materials used shall be a byproduct of the Plant, supplied by the Owner, and shall be loaded and hauled from the ansite silio ar stockpile area as directed by the Owner.
 2.2 All material used for construction of the Protection Layer shall be inert, non-expansive and pyrite free.
 2.3 All material be moved, dozed, bucketed into place by standard earthmoving equipment so as to establish compacted lifts of no more than 8 inches in overall thickness.
- thickness. 2.4 Each lift shall be compacted to a minimum of 75% relative density as determined by ASTM test No. D-4253 and D-4254. 2.5 The maximum particle size for any material to be incorporated into the Bottom Ash Protection Layer shall be 2 inches in its largest dimension, and no more than 5% by weight (of the 2 inch size material) may be utilized. 2.6 At no time shall more than 5% by weight of material smaller than the No. 200 U. S. Standard size be used in construction of the Bottom Ash Protection layer.

- 5. Standard sieve be used in construction of the bottom Ash Protection layer. 3. Granular Material (Underdrain Layer and Granular Roadway Surfacing) 3.1 All granular material shall be supplied from offsite sources by the Contractor. 3.2 Granular material shall be a natural gravel material conforming to gradation, material quality and durability requirements of ASHTO No. 57 coarse aggregate and 000T Construction and Material Specification Section 703. The material shall not be limestone. 3.3 All material shall be moved, dozed, bucketed into place by standard earthmoving equipment so as to establish compacted lifts of no more than 8 inches in overall thickness.
- equipment thickness.
- truckness. 3.4 Each lift shall be compacted to a minimum of 75% relative density as determined by ASTM test No. D-4253 and D-4254.

- Clay (Clay Liner Layer)
 All material required for the Compacted Clay Liner Layer shall be supplied by the Owner from on-site borrow areas, and shall be loaded and houled from the ansite areas as directed by the Owner.
 The Clay material shall meet the gradation and plasticity requirements for a Unified Soils Classification System soil type "CL".

- 4.2 The Clay material shall meet the gradution and plasticity requirements for a Unitied Soils Classification System soil type "CL".
 4.3 All material shall be moved, dozed, bucketed into place by standard earthmoving equipment so as to establish compacted lifts of no more than 6 inches in overall thickness. Clay layer lifts shall be placed at the same slope as the liner final grades.
 4.4 Prior to placement of a successive lift, the underlying material shall be scarified to a minimum depth of 2 inches. Only after the working surface is thoroughly scarified to a minimum depth of 2 inches. Only after the working surface is thoroughly scarified to a scale state site of the maximum dry density. for that the installed to take state ST the maximum dry density. for that so the state state is the state of the compacted shall be 0% to 4% above the optimum moisture content, as determined by the Standard tractor Test.
 4.7 The Owner shall perform a minimum of one in-place density and moisture content test (ASTM D-2488, D-422, D-424, D-2216 and D-854) per 5200 quore feet of compacted shall be 0%.
 4.8 The Owner shall perform a minimum of one in-place density and moisture content test (ASTM D-2222 and D-3017) per lift per 2500 square feet of compacted.
 4.9 The Owner shall perform a minimum of a Standard Practor Test (ASTM D-2922 and D-3017) per lift per 2500 square feet of compacted.
 4.9 The Owner shall perform a nimimum of a Standard Practor Test (ASTM D-298) for each different soil sample.
 4.0 The compaction specifications presented herein are considered the minimum compactive efforts. It shall be the Contractor's sole responsibility to place and compact the clay solis in such a fashin to provide an in-place permeability of 1 x 10-7 cm/sec or lower.
 4.11 All asomples shall be selected by the Owner and all testing shall be performed by an an advector of low of low and an advector so and an advector so and an advector so advector of low of low an
- 4.11 All samples shall be selected by the Owner and all testing shall be performed by an independent Laboratory contracted by the Owner.

FILL PLACEMENT, COMPACTION AND MATERIAL GRADATION NOTES

- 5. Common Fill Material 5.1 All material required for Common Fill shall be supplied by the Owner from on-site borrow areas, stockpiles or existing temporary dikes. Soil from on-site borrow areas shall be loaded and hauled from the ansite areas as directed by the Owner. Soil used as common fill shall be intert, free of organic material, debris and shall the function in the lorgest dimension.
 - on used as continuon nin shan be mert, nee ai organic material, deons and sin § Zinches in its lorgest dimension. I material shall be moved, dozed, bucketed into place by standard earthmoving jupment so as to establish compacted lifts of no more than 8 inches in overc
 - 5.3 Each lift shall be compacted to at least 95f the maximum dry density, for that
- 5.3 Each lift shall be compacted to at least 95 the maximum dry density, for that material, as determined by a Standard Proctor Test (ASTM-698). Should the Standard Proctor Test not be applicable due to the gradation of the material, then it shall be compacted to a minimum of 75% relative density tas determined by ASTM Test No. D-4253 and D-4254. ASTM Test No. D-4253 and D-4254. A The maisture content of the material to be compacted shall not deviate from the optimum mosture content, as determined by the Standard Proctor Test, by more than 25% relative hell and the processing of an Said (Classification Left (ASTM D-2005)).
- than 3%, mostate content, as determined by the database index to the set. By there than 5%, most the content of the set (ASTM D-2488, D-422, D-424, D-2216 and D-854) per \$000 cubic yords of soil, or for each different sail type, which wer results in a dynether fraquency.
 5.6 The Owner shall perform a minimum of one in-place density and moisture content test (KSTM D-292 and D-3017) per lift per 2500 square feet of composed soil, with a minimum of one test for ony day that sail material is compacted soil, with a minimum of one test for ony day that sail material is compacted soil, with a minimum of one test for ony day that sail material is compacted soil, and the owner shall perform a minimum of one Standard Proctor Test (ASTM D-698) for each different soil sample.
 5.8 All samples shall be selected by the owner and all testing shall be performed by an independent Laboratory contracted by the Owner.

- an independent Laboratory contracted by the Owner.
 6. Geotextile Filter Fabric
 6.1 All Geotextile Filter Fabric shall be Geotex type 451 nonwoven as manufactured by Synthetic Industries Inc. Geosynthetic Products Division. Roll width 12.5 or 15.0 feet and roll length \$00 feet 32 135 lbs.
 6.2 Grob Elongation ASIM D-4632 60 feet 33 75 lbs.
 7 Puncture Strength. ASIM D-4633 75 lbs.
 Mullen Burst ASIM D-4533 60 lbs.
 Apparent Opening size ASIM D-4751 100 U. S. Sieve Permitivity ASIM D-4491 1.20 sec-1
 Permeability ASIM D-4491 1.35 gam/ft2 UV resistance ASIM D-4557 70% retained © 500 hours
 6.3 All Geotextile Filter Fobric shall be stored, handled, installed and protected in accordance with the manufacturer's specifications.

- Stone Riprop Protection
 All material required for Stone Riprop Protection shall be supplied by the Contractor from off-site sources.
 Stone shall conform to the gradation and durability requirements of the Ohio Department of Transportation Type D "Dumped-Rock Fill".

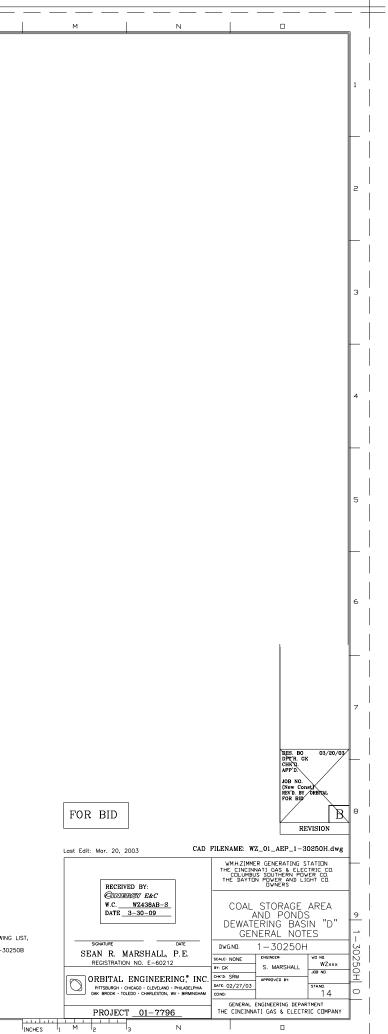
- ... stone snail conform to the gradation and durability requirements of the Ohio Department of Transportation Type D "Dumped-Rack Fill".
 8. Geomembrane
 8.1 Geomembrane Sheet shall be GSE HD Textured as Manufactured by GSE Lining Technology Inc. The material is a high density polyethylene. Roll width 22.5 feet x 325 ft.
 8.2 Thickness ASTM D-761/1593/5199 100 mils Density ASTM D-761/1593/5199 100 mils Density ASTM D-7638, Type IV Strength at Prack 125 lb/in-width Strength at Yield 216 lb/in-width Elongation at Bield 137 % Team resistance FTMS 101 130 Lb
 8.3 Geomembrane SASTM D-075 ib Puncture resistance FTMS 101 130 Lb
 8.4 The Geomembrane STMS 101 130 Lb
 8.4 The Geomembrane STMS shall be distructions.
 8.5 All Geomembrane material shall be stored, handled, installed and protected in accordance with the manufacturer's specifications.
 9.0 Erosion Control Blankets

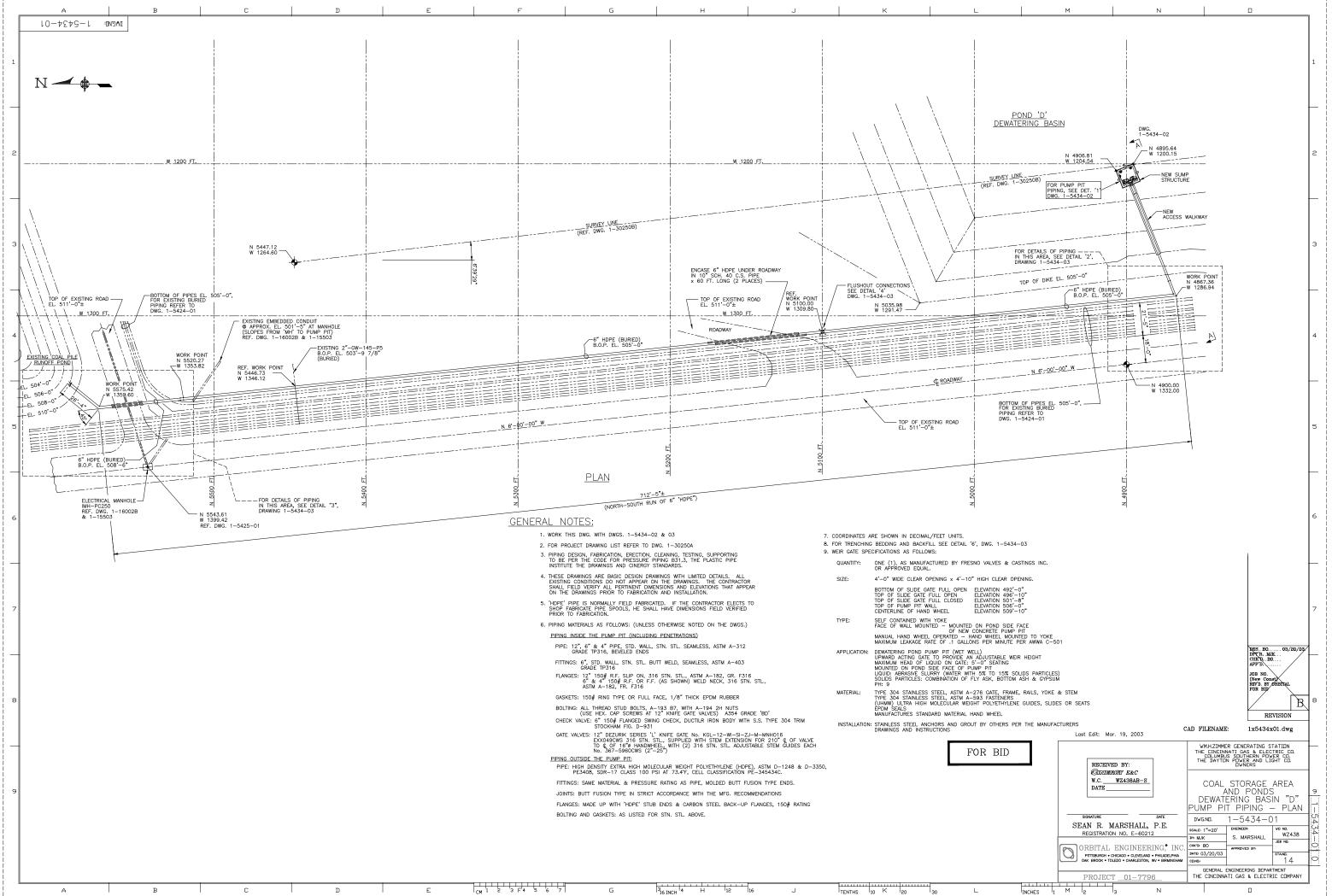
- 9.0 Erosion Control Blankets 9.1 The erosion control blanket shall be BioNet Series S150BN as Manufactured by North American Green. The blankets shall be 100% biodegradable consisting of agricultural straw fibers stitched bonded between 100% jute yorn nettings. Roll width 6.67 feet x 108 feet length. Stakes shall be 6° Eco transition between the state of the state nettings. Roll wi degradable type.

- Noti width 6.67 feet x 108 feet length. Stakes shall be 6° Eco degradable type.
 9.2 All erosion control blankets shall be stored, handled, installed and protected in accordance with the manufacturer's specifications.
 10.0 Erosion Control Revetment Fabric Forms for Concrete Fill
 10.1 The Erosion Control Revetment's shall be Fabriform Filter Point Style 8° FP as monactured by Construction Techniques Inc. The Fabric forms for labor down a store of 4° thickness manufactured tran double layer nylon fabric waven together in such a manor as to provide filter points on 8° centers for the relief of hydrostalic uplift pressures.
 10.2 Will width panels shall be shap assembled and sewn to 16° the site topography.
 10.3 Grout shall consist of a mixture of Portland cement, fine aggregate, admixtures and water so proportioned as to provide pumpable mix. The cured cement grout shall exhibit a minimum compressive strength of 3,000 psi in 28 days.
 11.0 Permovant Vacutifier

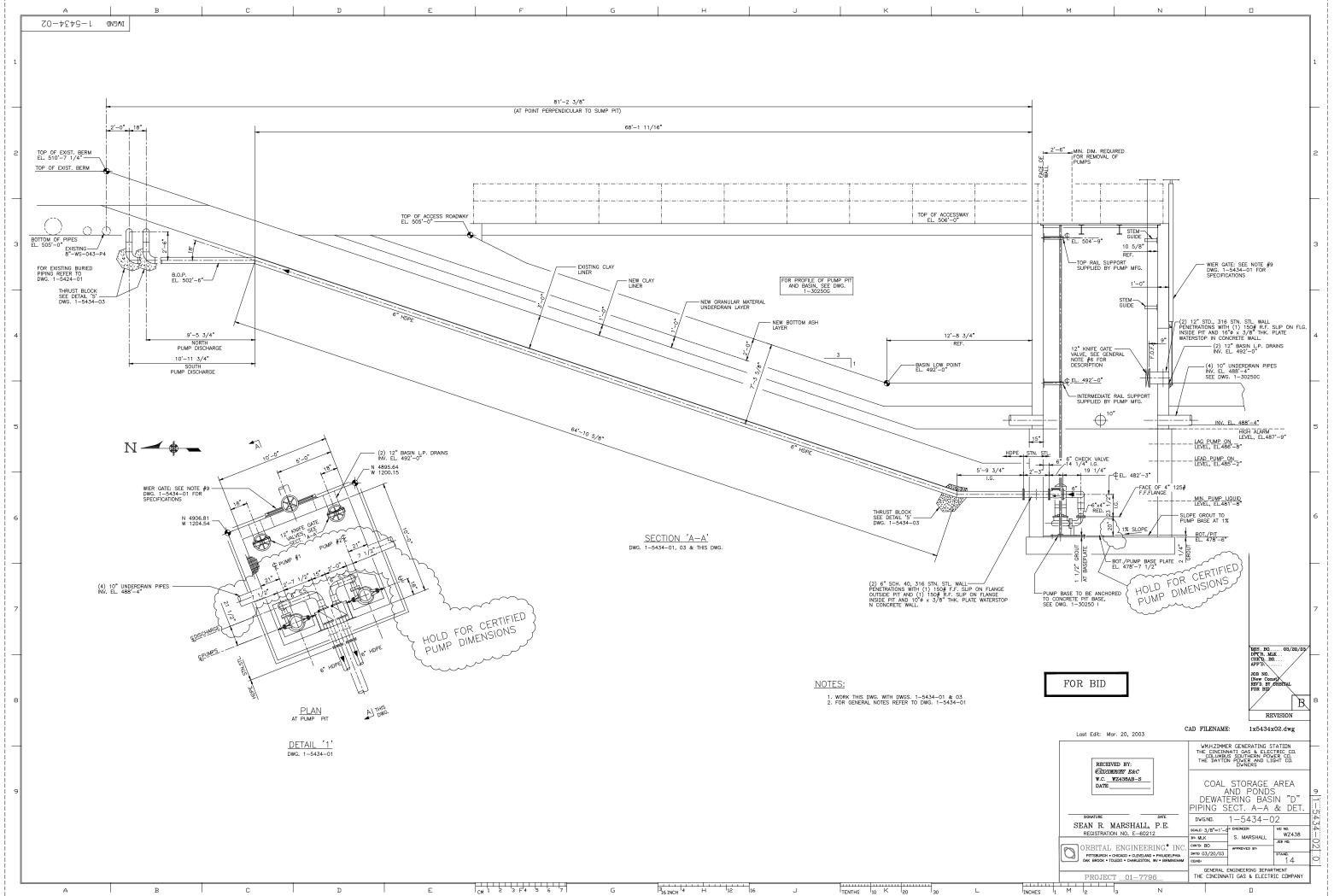
- 11.0 Permanent Vegetation 11.1 Temporary and Permanent Vegetation shall be performed on the outside surfaces of all new embankment slopes, disturbed portions of existing inside embankment slopes and existing roadway earth berms disturbed during construction.
- construction. 11.2 All Temporary and Permanent Vegetation shall be performed in accordance with the Clermont County Water Management & Sediment Control
- with the Clermont County Water Management & Sediment Control Regulations.
 11.3 Seedbed shall be prepared by roking and applying Lime and fertilizer. Lime acid soil (ph=5.5 or less) at a rate of 100 bs/1000 sf. Apply fertilizer at a rate of 12-15 bs/1000 sf of 10-10-10 or 12-12-12 analysis or equivalent.
 11.4 Seeding in the Spring with Tall Fescue at 1 b/1000 sf and a talter times Flatpea at 0.50 b & Tall Fescue at 0.50 b/1000 sf. Apply the seed uniformly by mechanical equipment or with tertilizer using a hydro-seed method.
 11.5 Inright with add legate unif growth is firmly established.
- - NOTE; 1. FOR PROJECT COVER SHEET & DRAWING LIST, SEE DWG. 1-30250A 2. WORK THIS DRAWING WITH DWGS. 1-30250B THROUGH 1-30250G

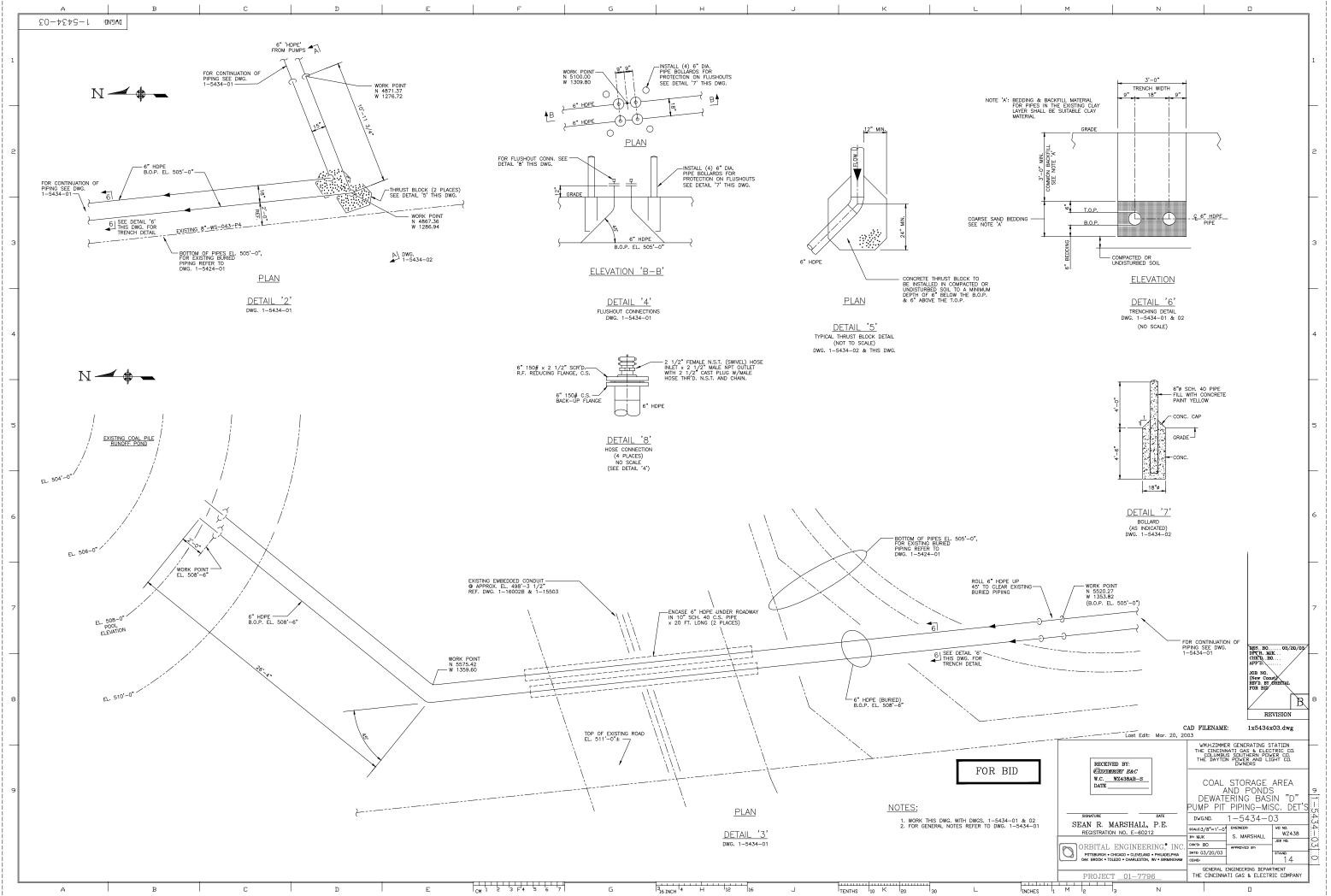
Т 1 2 3 F 4 5 6 7 316 INCH 4 H 12 16 TENTHS 10 K 20 30 В С D Е G J L





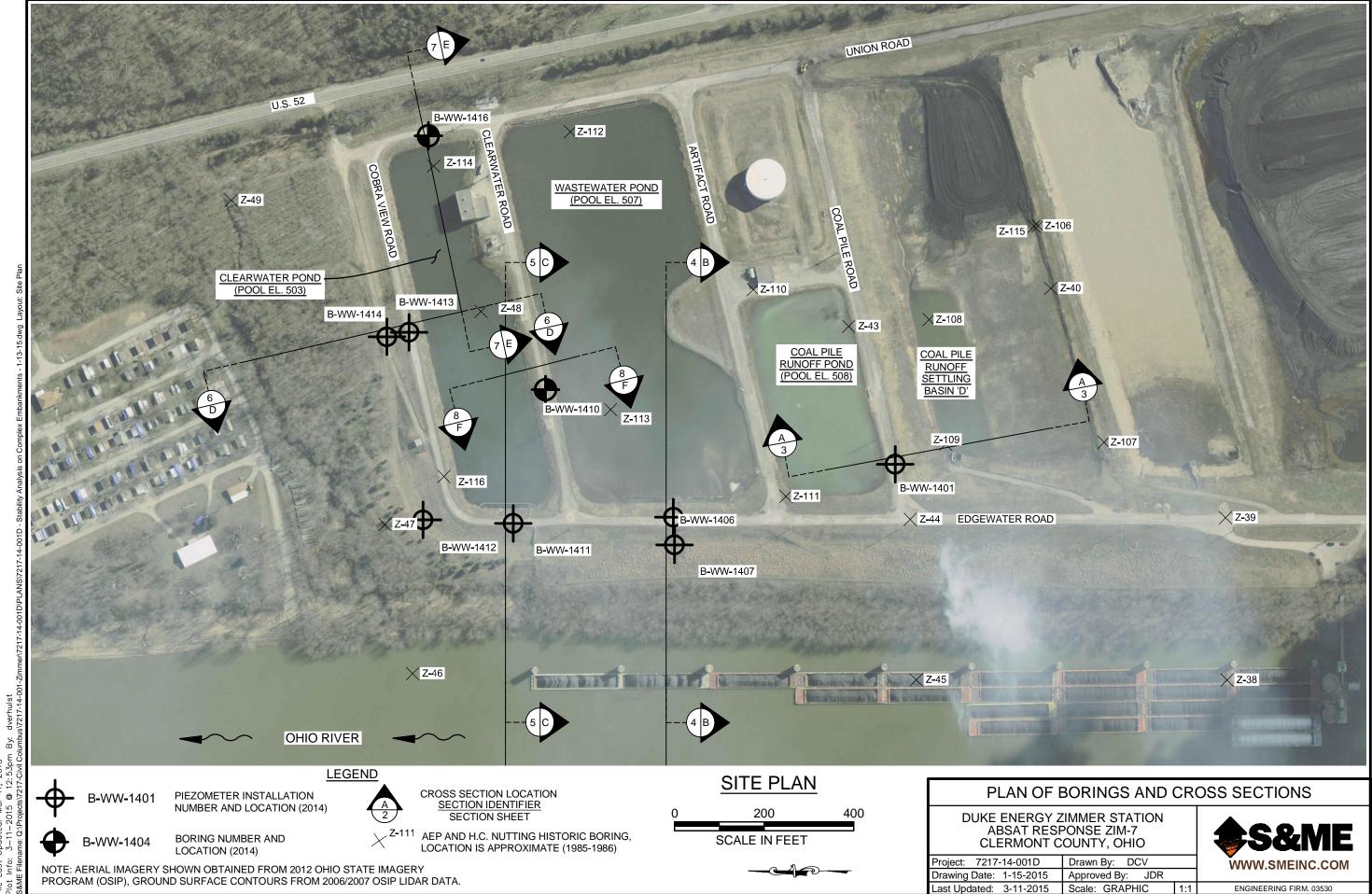
TENTHS Α В С D E G J







Appendix C: Zimmer Power Station Boring and Piezometer Locations



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